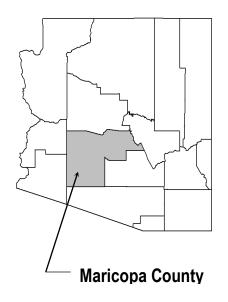


# MARICOPA COUNTY, ARIZONA AND INCORPORATED AREAS

COMMUNITY NAME	COMMUNITY NUMBER
AVONDALE, CITY OF	040038
BUCKEYE, CITY OF	040039
CAREFREE, TOWN OF	040126
CAVE CREEK, TOWN OF	040129
CHANDLER, CITY OF	040040
EL MIRAGE, CITY OF	040041
FOUNTAIN HILLS, TOWN OF	040135
GILA BEND, TOWN OF	040043
GILBERT, TOWN OF	040044
GLENDALE, CITY OF	040045
GOODYEAR, CITY OF	040046
GUADALUPE, TOWN OF	040111
LITCHFIELD PARK, CITY OF	040128
MARICOPA COUNTY	
(UNINCORPORATED AREAS)	040037
MESA, CITY OF	040048
PARADISE VALLEY, TOWN OF	040049
PEORIA, CITY OF	040050
PHOENIX, CITY OF	040051
QUEEN CREEK, TOWN OF	040132
SCOTTSDALE, CITY OF	045012
SURPRISE, CITY OF	040053
TEMPE, CITY OF	040054
TOLLESON, CITY OF	040055
WICKENBURG, TOWN OF	040056
YOUNGTOWN, TOWN OF	040057



PRELIMINARY MONTH XX, XXXX



# NOTICE TO FLOOD INSURANCE STUDY USERS

Communities participating in the National Flood Insurance Program have established repositories of flood hazard data for floodplain management and flood insurance purposes. This Flood Insurance Study (FIS) report may not contain all data available within the Community Map Repository. Please contact the Community Map Repository for any additional data.

The Federal Emergency Management Agency (FEMA) may revise and republish part or all of this FIS report at any time. In addition, FEMA may revise part of this FIS report by Letter of Map Revision process, which does not involve republication or redistribution of the FIS report. Therefore, users should consult community officials and check the Community Map Repository to obtain the most current FIS report components.

Users should refer to Section 10.0, Revisions Description, for further information. Section 10.0 is intended to present the most up-to-date information for specific portions of this FIS report. Therefore, users of this report should be aware that the information presented in Section 10.0 supersedes information in Sections 1.0 through 9.0 of the FIS report.

Initial Countywide FIS Effective Date: April 15, 1988

Revised Countywide Dates: September 29, 1989

September 4, 1991 December 3, 1993 September 30, 1995

July 19, 2001 September 30, 2005 October 16, 2013 November 4, 2015 Month XX, XXXX

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Upper Fan 5
Panels 1407P-1415P

<sup>\*</sup> Panel 1381P not printed

### **VOLUME 23**

<b>EXHIBITS</b> - continued	
Exhibit 1 - Flood Profiles - continued	
Valley Wash	Panels 1416P-1417P
Wagner Wash	Panels 1418P-1424P
Wagon Wash	Panel 1425P
Wash 1 East	Panels 1426P-1427P
Wash 1 West	Panels 1428P-1432P
Wash 2 East (North of the Central Arizona Project Canal)	Panels 1433P-1434P
Wash 2 East (South of the Central Arizona Project Canal)	Panels 1435P-1436P
Wash 2 East Tributary	Panels 1437P-1438P
Wash 2 West (North of the Central Arizona Project Canal)	Panels 1439P-1441P
Wash 2 West (South of the Central Arizona Project Canal)	Panels 1442P-1444P
Wash 2 West Tributary 1	Panels 1445P-1447P(e)
Wash 2 West Tributary 2	Panels 1448P-1450P
Wash 3 East	Panels 1451P-1455P
Wash 3 West	Panels 1456P-1461P
Wash 4 East	Panels 1462P-1463P
Wash 5 East	Panels 1464P-1467P
Wash 6 East	Panels 1468P-1470P
Wash 6 East South	Panel 1471P
Wash 7 East	Panel 1472P
Wash 7 East East Split	Panels 1473P-1474P
Wash 7 East Tributary	Panels 1475P-1476P
Wash 7 East West Split	Panel 1477P
Wash 8 East	Panels 1478P-1480P
Wash 9 (Rio Verde Wash 9)	Panels 1481P-1484P
Wash 9 East	Panels 1485P-1492P
Wash 9 East Split	Panel 1493P
Wash 10 East	Panels 1494P-1497P
Wash 10 East Split 1	Panel 1498P
Wash 10 East Split 2	Panels 1499P-1500P
Wash 11 East	Panels 1501P-1510P
Wash 12 East	Panels 1511P-1514P
Wash 12 East Split	Panel 1515P

#### **VOLUME 24**

#### **EXHIBITS** - continued

Wash 13 East Panels 1516P-1518P

Wash 14 East Panel 1519P

Wash AG
Panels 1520P(a)-1520P(d)
Wash B
Panels 1521P-1529P

Wash B Tributary Panel 1530P Mockingbird Wash Tributary 1 Panel 1531P

Wash F Panels 1532P(a)-1532P(c)

Wash F Tributary 1 Panel 1533P

 Wash G
 Panels 1534P(a)-1534P(b)

 Wash H
 Panels 1535P-1536P(c)

 Wash I
 Panels 1537P-1538P(b)

 Wash K
 Panels 1539P-1542P

 Wash K Tributary 1
 Panel 1543P(a)-1543P(b)

 Wash L
 Panels 1544P-1545P(c)

 Wash O
 Panels 1546P-1547P(c)

Wash P Panel 1548P

Wash Q Panels 1549P-1551P(c)

Wash S2 Panel 1552P

Wash T2N-R5W-S27N Panels 1553P-1555P Wash T4N-R2W-S09N Panels 1556P-1557P Wash T4N-R2W-S15N Panels 1558P-1559P Wash T4N-R3W-S07W Panels 1560P-1561P Wash T4N-R3W-S08E Panels 1562P-1565P Wash T4N-R3W-S08W Panels 1566P-1568P Wash T4N-R3W-S09W Panels 1569P-1571P Wash T4N-R3W-S10N Panels 1572P-1573P

Wash T4N-R3W-S10W-Reach-1 Panel 1574P Wash T4N-R3W-S10W-Reach-2 Panel 1575P

 Wash T4N-R3W-S17
 Panels 1576P-1578P

 Wash T4N-R3W-S18E
 Panels 1579P-1582P

 Wash T4N-R3W-S18W
 Panels 1583P-1586P

 Wash T5N-R2W-S07
 Panels 1587P-1588P

 Wash T5N-R2W-S19E
 Panels 1589P-1590P

 Wash T5N-R2W-S19W
 Panels 1591P-1594P

 Wash T5N-R3W-S01S
 Panel 1595P

 Wash T5N-R3W-S19
 Panel 1596P

Wash T5N-R3W-S24E Panels 1597P-1599P Waterfall Wash Panels 1600P-1607P

#### **VOLUME 25**

### **EXHIBITS** - continued

# Exhibit 1 - Flood Profiles - continued Waterman Wesh

Waterman Wash	Panels 1608P-1620P(ac)
West Fork White Peak Wash	Panel 1621P
West Garambullo Wash	Panels 1622P-1623P
West Quilotosa Wash	Panels 1624P-1625P
West Split Flow Through El Mirage	Panels 1626P-1627P
White Granite Wash	Panels 1628P-1633P
White Granite Wash North Fork	Panels 1634P-1636P
White Peak Wash	Panels 1637P-1639P
White Tanks No. 3 Wash	Panels 1640P-1646P
White Tanks Wash	Panels 1647P-1653P

#### **VOLUME 26**

#### **EXHIBITS** - continued

White Tanks Wash Tributary 1	Panels 1654P-1657P
Willow Springs Wash	Panels 1658P-1667P
Willow Springs Wash Tributary 1	Panels 1668P-1675P
Willow Springs Wash Tributary 1A	Panels 1676P-1679P
Willow Springs Wash Tributary 2	Panels 1680P-1683P
Willow Springs Wash Tributary 2A	Panels 1684P-1686P
Willow Springs Wash Tributary 4	Panels 1687P-1691P
Willow Springs Wash Tributary 5	Panels 1692P-1695P
Willow Springs Wash Tributary 5A	Panels 1696P-1698P
Willow Springs Wash Tributary 6	Panels 1699P-1701P
WILL GO TO WE I THE TO SEE	D 1 1500D

Willow Springs Wash Tributary 6A Panel 1702P
Willow Springs Wash Tributary 6B Panel 1703P
Willow Springs Wash Tributary 6C Panel 1704P

Windmill Wash Panels 1705P-1706P Windmill Wash North Branch Panels 1707P-1708P

Windmill Wash South Branch Panel 1709P

Wittmann Wash Panels 1710P-1721P

Wittmann Wash North Split Panel 1722P

Wittmann Wash South Split
Panels 1723P-1724P
Wittmann Wash Tributary
Panels 1725P-1726P
Yucca Flat Wash
Panels 1727P-1728P(i)

#### **VOLUME 27**

#### **EXHIBITS** - continued

Coyote Pass Wash	Panel 1/29P
Delaney Wash	Panels 1730P-1734P
Dalanay Wash North Split	Panel 1735P

Delaney Wash North Split Panel 1735P
Delaney Wash South Split Panel 1736P

Dickey Wash Panels 1737P-1741P
Eastern Canal Panels 1742P-1744P
Four Mile Wash Panels 1745P-1756P

Four Mile Wash W1 Panel 1757P Four Mile Wash W2 Panel 1758P

Gavilan Peak Wash Panels 1759P-1761P

Kelley Road Wash Panel 1762P

Lazy G Wash Panels 1763P-1764P

Luke Wash East Main Split Panel 1765P

Phillips Wash Panels 1766P-1771P
Photo View Wash Panels 1772P-1775P

Photo View Wash Breakout 1 Panel 1776P
Photo View Wash Breakout 2 Panel 1777P
Rio Verde Wash 7 Panels 1778P

Rio Verde Wash 10 Split 7 Tributary 1 Panels 1779P-1784P Rio Verde Wash 10 Tributary 1 Panels 1785P-1786P

Rio Verde Wash 10 Tributary 2

Rio Verde Wash 10 Tributary 2 Split 1

Rio Verde Wash 10 Tributary 3

Rio Verde Wash 10 Tributary 3

Panel 1789P

Rio Verde Wash 10 Tributary 4

Panel 1790P

Rio Verde Wash A Split 3 Panels 1791P-1793P

Rio Verde Wash A Split 4 Panel 1794P

Rio Verde Wash A Split 8

Rio Verde Wash A Split 9

Rio Verde Wash A Tank Spillway

Rio Verde Wash A Tank Spillway

Rio Verde Wash A Tributary 1

Rio Verde Wash A Tributary 2

Rio Verde Wash F Split 6

Rio Verde Wash F Tributary 2

Panels 1820P-1824P

<sup>\*</sup>Panel 1765P not printed

### **VOLUME 28**

### **EXHIBITS** - continued

Exhibit 1 - Flood Profiles - continued
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Rio Verde Wash K Tributary 6A

- 1 1000 1 1011165 - Collullaca	
Rio Verde Wash I	Panels 1825P-1839P
Rio Verde Wash I Split 4	Panels 1840P-1842P
Rio Verde Wash I Tributary 1	Panels 1843P-1845P
Rio Verde Wash I Tributary 3	Panel 1846P
Rio Verde Wash J	Panels 1847P-1849P
Rio Verde Wash K	Panels 1850P-1864P
Rio Verde Wash K Split 1	Panels 1865P-1866P
Rio Verde Wash K Split 3	Panels 1867P-1869P
Rio Verde Wash K Split 3A	Panels 1870P-1874P
Rio Verde Wash K Split 4	Panel 1875P
Rio Verde Wash K Tributary 1	Panels 1876P-1878P
Rio Verde Wash K Tributary 4	Panels 1879P-1888P
Rio Verde Wash K Tributary 4A	Panels 1889P-1894P
Rio Verde Wash K Tributary 6	Panels 1895P-1903P
Rio Verde Wash K Tributary 6 Split 1	Panels 1904P-1907P
Rio Verde Wash K Tributary 6 Split 2	Panels 1908P-1912P
Rio Verde Wash K Tributary 6 Split 3	Panels 1913P-1915P

Panels 1916P-1923P

#### **VOLUME 29**

EXHIBITS - continued	
Exhibit 1 - Flood Profiles - continued	
Rio Verde Wash K Tributary 6A1	Panels 1924P-1926P
Rio Verde Wash K Tributary 6A2	Panels 1927P-1928P
Rio Verde Wash K Tributary 6A3	Panel 1929P
Rio Verde Wash K Tributary 6B	Panels 1930P-1933P
Rio Verde Wash K Tributary 6C	Panel 1934P
Rio Verde Wash K Tributary 6D	Panels 1935P-1936P
Rio Verde Wash K Tributary 6D1	Panel 1937P
Rio Verde Wash K Tributary 7	Panels 1938P-1940P
Rio Verde Wash K Tributary 8	Panel 1941P
Rio Verde Wash K Tributary 9	Panels 1942P-1947P
Rio Verde Wash K Tributary 10	Panels 1948P-1949P
Rio Verde Wash K Tributary 11	Panels 1950P-1952P
Rio Verde Wash K Tributary 11A	Panels 1953P-1954P
Rio Verde Wash K Tributary 11B	Panel 1955P
Rio Verde Wash K Tributary 12	Panels 1956P-1957P
Rio Verde Wash K Tributary 13	Panels 1958P-1959P
Rio Verde Wash L	Panels 1960P-1967P
Rio Verde Wash P	Panels 1968P-1973P
Rio Verde Wash P Tributary 1	Panel 1974P
Rio Verde Wash P Tributary 2	Panel 1975P
River Creek	Panels 1976P-1977P
Rough Rider Wash	Panels 1978P-1980P
Sharman Wash	Panels 1981P-1982P
Soda Springs Wash	Panels 1983P-1984P
Table Mountain Wash	Panels 1985P-1987P
Table Mountain Wash Tributary 6	Panels 1988P-1989P
Twin Peaks Lane Wash	Panels 1990P-1991P
Wash T1N-R5W-S04	Panel 1992P
Wash T1N-R5W-S04 Split	Panel 1993P
Wash T1N-R5W-S10	Panels 1994P-1995P
Wash T1N-R5W-S15	Panels 1996P-1997P
Wash T1N-R5W-S18	Panel 1998P
Wash T1N-R5W-S22	Panels 1999P-2000P
Wash T1N-R5W-S28E	Panel 2001P
Wash T1N-R5W-S32	Panel 2002P
Wash T1N-R5W-S33E	Panel 2003P
Wash T1N-R5W-S33N	Panel 2004P

Panels 2005P-2006P

Wash T1N-R5W-S33W

#### **VOLUME 29 (Continued)**

#### **EXHIBITS** - continued

Exhibit 1 - Flood Profiles - continued

Wash T1N-R6W-S1 Panel 2007P

Wash T1N-R6W-S11 Panels 2008P-2009P

Wash T1N-R6W-S12 Panel 2010P

 Wash T1N-R6W-S17
 Panels 2011P-2012P

 Wash T1N-R6W-S18
 Panels 2013P-2014P

 Wash T1S-R2W-S32A (I63)
 Panels 2015P-2017P

Wash T1S-R5W-S09W Panel 2018P

Wash T1N-R6W-S1

### **VOLUME 30**

### **EXHIBITS** - continued

EXHIBITS - continued	
Exhibit 1 - Flood Profiles - continued	D 1 2010D 2020D
Wash T1S-R5W-S17	Panels 2019P-2028P
Wash T1S-R5W-S22N	Panels 2029P-2032P
Wash T1S-R5W-S22S	Panels 2033P-2035P
Wash T1S-R5W-S29	Panels 2036P-2040P
Wash T1S-R5W-S29W	Panel 2041P
Wash T1S-R6W-S05S	Panels 2042P-2043P
Wash T1S-R6W-S08	Panels 2044P-2048P
Wash T1S-R6W-S27	Panels 2049P-2050P
Wash T2N-R5W-S27S	Panels 2051P-2052P
Wash T2N-R5W-S28	Panel 2053P
Wash T2N-R5W-S32	Panel 2054P
Wash T2N-R5W-S33E	Panels 2055P-2057P
Wash T2N-R5W-S33W	Panel 2058P
Wash T2N-R6W-S02	Panels 2059P-2060P
Wash T2N-R6W-S05E	Panels 2061P-2064P
Wash T2N-R6W-S05N	Panels 2065P-2068P
Wash T2N-R6W-S05S	Panel 2069P
Wash T2N-R6W-S05W	Panel 2070P
Wash T2N-R6W-S22	Panels 2071P-2072P
Wash T2N-R6W-S28N	Panels 2073P-2076P
Wash T2N-R6W-S36	Panels 2077P-2078P
Wash T2N-R6W-S36W	Panels 2079P-2080P
Wash T2N-R7W-S20W	Panels 2081P-2082P
Wash T2N-R7W-S32E	Panels 2083P-2084P
Wash T2N-R7W-S35W	Panels 2085P-2088P
Wash T3N-R6W-S27W	Panels 2089P-2090P
Wash T3N-R6W-S32	Panel 2091P
Wash T3N-R6W-S33	Panel 2092P
Wash T3N-R6W-S35	Panel 2093P
Wash T5N-R3W-S15-1-1E (West Fork Trilby Wash	
Tributary 1 East)	Panels 2094P-2095P
Wash T5N-R3W-S15-1E (Trilby Wash Tributary 1 East)	Panels 2096P-2098P
Wash T5N-R3W-S28-3W (Iona Tributary 3 West)	Panels 2099P-2102P
Wash T5N-R4W-S3	Panels 2103P-2104P
Wash T5N-R4W-S7A	Panels 2105P-2106P
Wash T5N-R4W-S7B	Panel 2107P
Wash T5N-R4W-S7C	Panels 2108P-2109P
Wash T5N-R4W-S19	Panels 2110P-2112P

#### **VOLUME 30 (Continued)**

### **EXHIBITS** - continued

Exhibit 1 - Flood Profiles - continued Wash T5N-R4W-S20A Wash T5N-R4W-S20B

Panels 2113P-2118P Panels 2119P-2120P

### **VOLUME 31**

### **EXHIBITS** - continued

EXHIBITS - continued	
Exhibit 1 - Flood Profiles - continued	D 1 0101D 0105D
Wash T5N-R4W-S21	Panels 2121P-2125P
Wash T5N-R4W-S30	Panels 2126P-2127P
Wash T5N-R5W-S1	Panel 2128P
Wash T5N-R5W-S3A	Panel 2129P
Wash T5N-R5W-S3B	Panel 2130P
Wash T5N-R5W-S10A	Panels 2131P-2132P
Wash T5N-R5W-S11	Panel 2133P
Wash T5N-R5W-S12	Panel 2134P
Wash T5N-R5W-S13A	Panel 2135P
Wash T5N-R5W-S13B	Panels 2136P-2137P
Wash T5N-R5W-S14	Panel 2138P
Wash T5N-R5W-S14B	Panel 2139P
Wash T5N-R5W-S22	Panel 2140P
Wash T5N-R5W-S23A	Panels 2141P-2142P
Wash T5N-R5W-S23B	Panels 2143P-2146P
Wash T5N-R5W-S23C	Panel 2147P
Wash T5N-R5W-S23D	Panel 2148P
Wash T5N-R5W-S23E	Panel 2149P
Wash T5N-R5W-S23F	Panel 2150P
Wash T5N-R5W-S25A	Panels 2151P-2155P
Wash T5N-R5W-S25B	Panels 2156P-2161P
Wash T5N-R5W-S25C	Panels 2162P-2167P
Wash T5N-R5W-S34C	Panels 2168P-2171P
Wash T5N-R5W-S35	Panels 2172P-2175P
Wash T6N-R4W-S33	Panel 2176P
Wash T6N-R4W-S33A	Panel 2177P
Wash T6N-R5W-S36	Panel 2178P
Wash T6N-R5W-S36A	Panel 2179P
Wash T6N-R5W-S36B	Panel 2180P
West Prong of Waterman Wash	Panels 2181P-2183P
White Spar Wash	Panels 2184P-2185P
Winters Wash	Panels 2186P-2192P
Winters Wash With Embankment	Panel 2193P
Wittmann Wash Tributary 1	Panels 2194P-2198P
Wittmann Wash Tributary 1 Breakout 1	Panel 2199P
Wittmann Wash Tributary 1 Breakout 1 of Breakout 3	Panel 2200P
Wittmann Wash Tributary 1 Breakout 2	Panel 2201P
Wittmann Wash Tributary 1 Breakout 3	Panel 2202P

#### **VOLUME 31 (Continued)**

#### **EXHIBITS** - continued

#### Exhibit 1 - Flood Profiles - continued

Wittmann Wash Tributary 1 Breakout 4	Panels 2203P-2204P
Iona Tributary 1 West	Panels 2205P-2208P
Iona Tributary 2 West	Panels 2209P-2210P
W. J. F.I.G. DOWL GLOAT (197)	D 1 2211D

Wash T1S-R2W-S18A (J27) Panel 2211P

Wash T1S-R2W-S18B (J37) Panels 2212P-2213P Wash T1S-R2W-S31 (A56) Panels 2214P-2216P

Wash T1S-R2W-S31B (I70) Panel 2217P Wash T1S-R3W-S24A (A60) Panel 2218P

Wash T2S-R2W-S7A (A52)
Wash T2S-R2W-S7B (A51)
Panels 2221P-2222P
Amir Wash Tributary 1
Panels 2223P-2225P
Amir Wash Tributary 2
Panels 2226P-2227P
Amir Wash Tributary 3
Panels 2228P-2229P

Casandro Wash Southwest Split Panel 2230P
Casandro Wash Val Vista Split Panel 2231P
Cemetery Wash Tributary R Panel 2232P

Cemetery Wash Tributary R-2A Panels 2233P-2234P
Cemetery Wash Tributary R-4 Panels 2235P-2238P

#### **VOLUME 32**

### **EXHIBITS** - continued

Errhihit	1 E	Loof.	Drofil.	00	continued
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ibit 1 -	- Flood Profiles - continued	
	Centennial Wash Field Overflow (Without Embankment)	Panel 2239P
	Centennial Wash West Railroad Overflow (Without	
	Embankment)	Panel 2240P
	Flying E Wash Split	Panel 2241P
	Flying E Wash Tributary 1	Panels 2242P-2243P
	Flying E Wash Tributary 2	Panels 2244P-2245P
	Flying E Wash Tributary 3	Panels 2246P-2249P
	Flying E Wash Tributary A	Panel 2250P
	Harquahala Drainage Channel	Panels 2251P-2252P
	Hartman Wash Breakout	Panel 2253P
	Hartman Wash Split	Panel 2254P
	Hartman Wash Tributary 2	Panels 2255P-2257P
	Little San Domingo Wash Tributary 1	Panels 2258P-2262P
	Powder House Wash Side Channel	Panel 2263P
	Sols Wash Tributary 1S	Panels 2264P-2266P
	Sols Wash Tributary 2S	Panels 2267P-2269P
	Wash AF	Panels 2270P-2271P
	Wash HT07	Panels 2272P-2278P
	Wash J	Panels 2279P-2281P
	Wash M	Panels 2282P-2284P
	Wash N	Panels 2285P-2290P
	Yucca Flat Wash Tributary 1	Panel 2291P

#### **PUBLISHED SEPARATELY**

Flood Insurance Rate Map Index Flood Insurance Rate Map

#### 3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of flooding from the sources studied were carried out to provide estimates of the elevations of floods of the selected recurrence intervals. Users should be aware that flood elevations shown on the Flood Insurance Rate Map (FIRM) represent rounded whole-foot elevations and may not exactly reflect the elevations shown on the Flood Profiles or in the Floodway Data tables in the Flood Insurance Study (FIS) report. Flood elevations shown on the FIRM are primarily intended for flood insurance rating purposes. For construction and/or floodplain management purposes, users are cautioned to use the flood elevation data presented in this FIS in conjunction with the data shown on the FIRM.

For areas of riverine flooding studied by detailed methods, water-surface elevations for floods of the selected recurrence intervals were computed using the U.S. Army Corps of Engineers (USACE) HEC-2 computer program (U.S. Department of the Army, 1973).

The cross section data for the Agua Fria River were taken from several sources of mapping. A 1981 USACE topographic map for the New River (U.S. Department of the Army, 1981) was used for the river section from the confluence with the Gila River to the confluence with the New River from the New River to Northern Avenue, 1982 City of Glendale mapping was used (City of Glendale, 1982). From Northern Avenue to Grand Avenue and from Beardsley Road to Jomax Road, 1983 Maricopa County maps were used (Maricopa County, 1983). The topographic maps for the reach between Grand Avenue and Bell Road (American Engineering Company, 1982) were furnished by American Engineering Company for the reach between Bell and Beardsley Roads, maps were provided by Cella, Barr, Evans and Associates (Cella Barr Evens & Associates, 1982).

Cross sections for the Gila River were digitized from 1983 topographic maps or taken from as-built data for the Bullard Avenue Bridge.

Cross sections for the Salt River between Central Avenue and 115th Avenue were based on digitized data from topographic mapping. From Central Avenue to Country Club Road in the City of Mesa, cross sections were also taken from topographic mapping (Arizona Department of Transportation, 1982; City of Phoenix, 1983).

For study purposes, Skunk Creek was divided into two sections. Lower Skunk Creek lies between Adobe Dam outlet channel and the sections were also taken from topographic mapping (Arizona Department of Transportation, 1982) Bell Road Bridge. Upper Skunk Creek is from the Central Arizona with a contour interval of 2 feet.

Project channel to Adobe Dam. Cross sections for both reaches were generated using 1974 Maricopa County topographic maps at a scale of 1:2,400, with a contour interval of 2 feet. These maps were supplemented by additional mapping from the City of Phoenix and the USACE at scales of 1:1,200 and 1:2,400, respectively, both cross sections for the Hassayampa River (below Carefree Highway) were field surveyed.

Cross-section data for the following were developed from topographic maps (Harris-Toups Associates, 1976): Skunk Creek above Carefree Highway; Cave Creek above Cave

Creek Dam; Andora Hills, Galloway, Rowe, Grapevine, Ocotillo, Willow Springs, Powder House, Mockingbird, and Little San Domingo Washes; Whitman Drainage; Aguila Farm Channel; Grass, Sand Tank, and Bender Washes; Rodeo Wash and its tributary; Airport, and Scott Avenue Washes; Lower El Mirage Wash and its tributary; Atchison, Topeka & Santa Fe Railway Channel at El Mirage; the Atchison, Topeka & Santa Fe Railway at Peoria; and the Southern Pacific Railroad and its spurs.

Cross-section data for East Branch Scatter Wash and Echo Canyon Washes were developed from topographic maps provided by the City of Phoenix (City of Phoenix, 1967).

Cross-section data for Cave Creek below Arizona Canal and for East Fork of Cave Creek were developed from aerial photographs flown in March 1980 (Aerial Mapping of Phoenix, Arizona, 1980). Cross-section data for Cave Creek between Arizona Canal and Cave Creek Dam were developed from aerial photographs flown in March 1978 (Aerial Mapping of Phoenix, Arizona, 1978).

Cross-section data for the Sols Wash backwater analyses were obtained from topographic maps, at a scale of 1:200, with a contour interval of 2 feet, prepared specifically for this project by Cooper Aerial Survey in March 1986 (Cooper Aerial Survey, 1986). Culvert and bridge data were obtained from the topographic maps and were field checked to verify structural geometry.

Cross-section data for Casandro, South Branch Casandro, Flying E, and Hospital Washes were taken from a USACE Flood Plain Information report for Wickenburg (U.S. Department of the Army, 1965) and from topographic maps (U.S. Department of the Army, 1976).

Cross-section data for Martinez Wash were digitized from topographic maps (U.S. Department of the Army, 1968).

Cross sections were located at close intervals above and below bridges in order to compute the significant backwater effects of these structures. All bridges and culverts were investigated to obtain elevation data and structural geometry.

Locations of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles (Exhibit 1). For stream segments for which a floodway was computed (Section 4.2), selected cross section locations are also shown on the Flood Boundary and Floodway Map (Exhibit 2).

Starting water-surface elevations for all riverine flooding sources, except as noted below, were developed using the slope/area method.

The starting water-surface elevations for the New River were developed through the use of 1985 topographic mapping in the area of its confluence with Skunk Creek. A significant feature of the New River floodplain is the channelization in the vicinity of its confluence with Skunk Creek. This channelization has occurred from approximately 1,500 feet downstream of the Thunderbird Road Bridge upstream to the Greenway Road.

In addition, in the left over bank area above Union Hills Drive, a new wastewater treatment plant with improved channel banks is reflected in the hydraulic model.

For the upper reaches of Skunk Creek, the starting water-surface elevations were computed from the reservoir spillway elevation of 1,377 feet. For the lower reach, normal-depth and New River backwater computations were used.

Hydraulic roughness coefficients (Manning's "n") were selected on the basis of field inspection and engineering judgment. Table 5, "Range of Hydraulic Roughness Coefficients (Manning's "n")" gives the range of Manning's "n" values for each flooding source studied by detailed methods.

Table 5. Range of Hydraulic Roughness Coefficients (Manning's "n")

Flooding Source	<b>Channel</b>	<b>Overbanks</b>
191st Avenue Wash	0.012 - 0.030	0.040 - 0.070
Agua Fria River	0.022 - 0.059	0.032 - 0.070
Aguila Farm Channel	0.030	0.040 - 0.050
Airport Wash	0.025	0.035
Andora Hills Wash	0.020 - 0.045	0.020- 0.052
Apache Wash	0.045 - 0.060	0.070
Apache Wash West Fork	0.045 - 0.060	0.070
Atchison, Topeka & Santa Fe Railway Channel	0.032 - 0.037	0.032 - 0.047
Atchison, Topeka & Santa Fe (AT&SF) Railway Channel	0.035 - 0.045	0.030 - 0.080
Atchison, Topeka & Santa Fe Railway Ponding	0.035 - 0.040	0.035 - 0.040
Beardsley Canal Wash	0.024 - 0.035	0.024 - 0.070
Bedrock Wash	0.045 - 0.050	0.035 - 0.070
Bender and Sand Tank Washes	0.025	0.035
Bender Wash	0.025 - 0.080	0.030 - 0.035
Bullard Wash	0.013 - 0.070	0.030 - 0.070
Bulldozer Wash	0.035 - 0.050	0.040 - 0.070
Casandro Wash	0.030 - 0.060	0.040 - 0.060
Casandro Wash South Branch	0.030 - 0.060	0.040 - 0.060
Cave Creek	0.015 - 0.065	0.035 - 0.065

Table 5. Range of Hydraulic Roughness Coefficients (Manning's "n") (Continued)

Flooding Source	<b>Channel</b>	<b>Overbanks</b>
Cemetery Wash	0.035 - 0.100	0.040 - 0.100
Centennial Wash	0.030 - 0.070	0.030 - 0.200
Centennial Wash	0.040	0.040
Cholla Wash	0.035 - 0.070	0.030 - 0.070
Circle City Area Washes	0.030 - 0.080	0.030 - 0.080
Cline Creek and Tributaries	0.045 - 0.075	0.045 - 0.080
Consolidated Canal, Ponding	0.025 - 0.075	0.025 - 0.075
Cottonwood Creek	0.030 - 0.060	0.050 - 0.080
Cottonwood Creek Tributary 1	0.045 - 0.050	0.060 - 0.070
Cottonwood Creek Tributary 2	0.050	0.060 - 0.070
Coyote Pass Wash	0.035 - 0.06	0.025 - 0.5
Dale Creek	0.025 - 0.035	0.025 - 0.050
Delaney Wash	0.046 - 0.065	0.02 - 0.065
Delaney Wash North Split	0.046 - 0.065	0.042 - 0.065
Delaney Wash South Split	0.042 - 0.065	0.02 - 0.065
Desert Hills Wash	0.012 - 0.055	0.050 - 0.114
Desert Hills Wash - West Branch	0.050 - 0.060	0.052 - 0.065
Desert Lake Wash	0.050	0.060 - 0.065
Dickey Wash	0.05 - 0.06	0.045 - 0.06
Diversion Dike Wash	0.035	0.035 - 0.070
East Fork of Cave Creek	0.015 - 0.035	0.035 - 0.045
East Garambullo Wash	0.024 - 0.055	0.036 - 0.060
Eastern Canal, Ponding	0.032 - 0.075	0.032 - 0.075
Echo Canyon Wash	0.018 - 0.025	0.012 - 0.035
Fleming Springs Wash	0.038 - 0.060	0.055 - 0.060
Flying E Wash	0.030 - 0.060	0.040 - 0.060
Four Mile Wash	0.02 - 0.075	0.02 - 0.075

Table 5. Range of Hydraulic Roughness Coefficients (Manning's "n") (Continued)

Flooding Source	Channel	Overbanks
Four Mile W1	0.037	0.037
Four Mile W2	0.035 - 0.042	0.035 - 0.042
Galloway Wash	0.032 - 0.045	0.016 - 0.045
Galloway Wash	0.032 - 0.045	0.016 - 0.045
Galloway Wash-North Tributary 1	0.025 - 0.040	0.045
Gavilan Peak Wash	0.025 - 0.06	0.025 - 0.5
Gila Bend Canal	0.045	0.050
Gila River	0.030 - 0.120	0.035 - 1.000
Grapevine Wash	0.020 - 0.052	0.020 - 0.046
Grass Wash	0.025 - 0.040	0.025 - 0.045
Hassayampa River	0.030 - 0.050	0.030 - 1.000
Hospital Wash	0.030 - 0.060	0.040 - 0.060
Interstate 10	0.035	0.045 - 0.050
Jackrabbit Trail	0.012 - 0.030	0.012 - 0.060
Jackrabbit Wash	0.030 - 0.035	0.035 - 0.040
Kelley Road Wash	0.035 - 0.06	0.025 - 0.5
Lazy G Wash	0.035 - 0.055	0.055
Little San Domingo Wash	0.030	0.040
Lower El Mirage Wash	0.044	0.044
Lower El Mirage Wash	0.030 - 0.045	0.035 - 0.100
Lower El Mirage Wash Tributary	0.040 - 0.045	0.070 - 0.100
Lower El Mirage Wash Tributary	0.044	0.044
Luke Wash	0.045 - 0.075	0.045 - 0.075
Luke Wash (For concrete box culverts)	0.017	N/A
Luke Wash East Main Split	0.045	0.05
Luke Wash - East Main Tributary	0.045 - 0.06	0.050 - 0.120
Luke Wash - East Sub-Tributary	0.045 - 0.055	0.045 - 0.06

 Table 5. Range of Hydraulic Roughness Coefficients (Manning's "n") (Continued)

Flooding Source	<b>Channel</b>	<u>Overbanks</u>
Luke Wash - Minor Tributary	0.045	0.05
Martinez Wash	0.025 - 0.060	0.060 - 0.100
McMicken Dam Outlet Wash	0.020 - 0.050	0.035 - 0.080
Mesquite Tank Wash	0.060	0.070
Mockingbird Wash	0.030 - 0.037	0.035 - 0.042
Morgan City Wash	0.035 - 0.100	0.055 - 0.100
New River	0.030 - 0.035	0.030 - 0.060
Centennial Wash North Branch	0.040	0.040
North Fork Bedrock Wash	0.035 - 0.045	0.070
North Fork Cholla Wash	0.070	0.070
North Fork White Granite Wash	0.035	0.070
Ocotillo Wash	0.020 - 0.045	0.020 - 0.052
Ocotillo Wash Tributary 1	0.035 - 0.040	0.045
Ocotillo Wash Tributary 1A	0.032 - 0.035	0.040 - 0.045
Ocotillo Wash Tributary 2	0.035 - 0.045	0.040 - 0.050
Ocotillo Wash Tributary 3	0.045 - 0.055	0.055
Ocotillo Wash Tributary 4	0.025 - 0.045	0.045 - 0.050
Osborn Road Wash	0.030 - 0.035	0.050 - 0.070
Paradise Wash	0.013 - 0.055	0.050 - 0.070
Paradise Wash - West Branch	0.050 - 0.055	0.053 - 0.065
Perryville Road Wash	0.022 - 0.045	0.035 - 0.080
Phillips Wash	0.04 - 0.06	0.045 - 0.06
Photo View Wash	0.038 - 0.55	0.025 - 0.06
Photo View Wash Breakout 1	0.038 - 0.055	0.055
Photo View Wash Breakout 2	0.055	0.055 - 0.5
Photo View Wash Breakout 3	0.055	0.055 - 0.5
Powder House Wash	0.030 - 0.060	0.040 - 0.060

Table 5. Range of Hydraulic Roughness Coefficients (Manning's "n") (Continued)

Flooding Source	<b>Channel</b>	<b>Overbanks</b>
Powerline Wash	0.040 - 0.045	0.050 - 0.055
Powerline Wash	0.040 - 0.041	0.043 - 0.055
Rainbow Wash	0.016 - 0.047	0.030 - 0.150
Rainbow Wash Tributary	0.013 - 0.040	0.040 - 0.050
Ranieri Wash	0.050	0.065
Rio Verde Wash 7	0.035 - 0.065	0.045 - 0.065
Rio Verde Wash 10	0.035 - 0.055	0.045 - 0.055
Rio Verde Wash 10 Tributary 1	0.035 - 0.06	0.035 - 0.06
Rio Verde Wash 10 Tributary 2	0.045 - 0.055	0.045 - 0.055
Rio Verde Wash 10 Tributary 2 Split 1	0.045 - 0.055	0.045 - 0.055
Rio Verde Wash 10 Tributary 3	0.045 - 0.055	0.045 - 0.055
Rio Verde Wash 10 Tributary 4	0.045 - 0.055	0.045 - 0.055
Rio Verde Wash A	0.04 - 0.05	0.04 - 0.05
Rio Verde Wash A Tank Spillway	0.04 - 0.05	0.04 - 0.05
Rio Verde Wash A Tributary 1	0.05 - 0.075	0.05 - 0.075
Rio Verde Wash A Tributary 2	0.04 - 0.065	0.04 - 0.065
Rio Verde Wash I	0.05 - 0.055	0.05 - 0.055
Rio Verde Wash I Tributary 3	0.05 - 0.055	0.05 - 0.055
Rio Verde Wash K	0.04 - 0.06	0.04 - 0.06
Rio Verde Wash K Split4	0.04 - 0.06	0.04 - 0.06
Rio Verde Wash K Tributary 6	0.04 - 0.055	0.04 - 0.055
Rio Verde Wash K Tributary 6D	0.05 - 0.055	0.05 - 0.055
Rio Verde Wash K Tributary 6D1	0.045 - 0.06	0.055 - 0.06
Rio Verde Wash K Tributary 6D Split 1	0.05 - 0.055	0.055
Rio Verde Wash K Tributary 11	0.045	0.045
Rio Verde Wash K Tributary 11A	0.045	0.045
Rio Verde Wash K Tributary 11B	0.04 - 0.045	0.045

 Table 5. Range of Hydraulic Roughness Coefficients (Manning's "n") (Continued)

Flooding Source	<b>Channel</b>	<u>Overbanks</u>
Rio Verde Wash K Tributary 12	0.045 - 0.05	0.045 - 0.05
Rio Verde Wash K Tributary 13	0.04 - 0.06	0.04 - 0.06
Rio Verde Wash P	0.04 - 0.055	0.045 - 0.055
Rio Verde Wash P Tributary 1	0.05 - 0.055	0.05 - 0.055
Rio Verde Wash P Tributary 2	0.05 - 0.055	0.05 - 0.055
River Creek	0.038 - 0.055	0.05 - 0.055
Rodeo Wash	0.025	0.035
Rodeo Wash Tributary	0.025	0.035
Rodger Creek	0.045 - 0.080	0.055 - 0.080
Rough Rider Wash	0.03 - 0.055	0.025 - 0.5
Rowe Wash	0.020 - 0.045	0.020 - 0.052
Rowe Wash Tributary 1	0.045	0.045 - 0.055
Rowe Wash Tributary 2	0.045	0.050 - 0.055
Salt River	0.030 - 0.035	0.040 - 0.050
Sand Tank Wash	0.025 - 0.080	0.035 - 0.060
Scatter Wash, North Branch	0.020 - 0.050	0.070 - 0.150
Scatter Wash, South Branch	0.035	0.045
Scott Avenue Wash	0.025 - 0.080	0.035
Scott Avenue Wash	0.035 - 0.080	0.035 - 0.040
Sharman Wash	0.025 - 0.06	0.025 - 0.06
Skunk Creek	0.020 - 0.050	0.045 - 0.050
Soda Springs Wash	0.025 - 0.06	0.025 - 0.5
Sols Wash	0.035 - 0.065	0.025 - 0.100
South Branch of Tank Wash	0.040 - 0.050	0.050 - 0.055
Southern Pacific Railroad	0.014 - 0.050	0.014 - 0.100
Southern Pacific Railroad & Southern Pacific Spur, Ponding	0.025 - 0.075	0.025 - 0.075
Stagecoach Pass Wash Overflow	0.030	0.040

Table 5. Range of Hydraulic Roughness Coefficients (Manning's "n") (Continued)

Flooding Source	<u>Channel</u>	<u>Overbanks</u>
Star Wash	0.030 - 0.035	0.035 - 0.040
Star Wash	0.036 - 0.044	0.043 - 0.045
Sweat Canyon Wash	0.035	0.055
Table Mountain Wash	0.03 - 0.06	0.025 - 0.5
Table Mountain Wash Tributary 6	0.038 - 0.055	0.055
Tank Wash	0.040 - 0.041	0.043 - 0.055
Tank Wash	0.040 - 0.046	0.050 - 0.055
Tractor Wash	0.030 - 0.035	0.035 - 0.075
Tributary A	0.042 - 0.044	0.042
Tributary B	0.042	0.045
Tributary C	0.040	0.045
Tributary D	0.038 - 0.040	0.044 - 0.045
Tributary E	0.038 - 0.040	0.044
Trilby Wash	0.040 - 0.100	0.050 - 0.100
Tuthill Dike Wash	0.016 - 0.030	0.016 - 0.050
Twin Buttes Wash	0.024 - 0.055	0.036 - 0.060
Twin Peaks Lane Wash	0.025 - 0.055	0.025 - 0.5
Union Pacific Railroad	0.035	.035 - 0.05
Unnamed Tributary of Jackrabbit Wash	0.030 - 0.035	0.035 - 0.040
Unnamed Wash No. 1	0.025 - 0.080	0.030 - 0.035
Unnamed Wash No. 2	0.025 - 0.080	0.030 - 0.070
Wagner Wash	0.040 - 0.105	0.065 - 0.100
Wash 2 West Tributary 1	0.033 - 0.082	0.033 - 0.082
Wash T1N-R5W-S04	0.06	0.055
Wash T1N-R5W-S04Split	0.06	0.055
Wash T1N-R5W-S10	0.045	0.055
Wash T1N-R5W-S15	0.05	0.065

Table 5. Range of Hydraulic Roughness Coefficients (Manning's "n") (Continued)

Flooding Source	<b>Channel</b>	<u>Overbanks</u>
Wash T1N-R5W-S18	0.045	0.06
Wash T1N-R5W-S22	0.05-0.065	0.065
Wash T1N-R5W-S28E	0.045	0.05
Wash T1N-R5W-S32	0.045	0.05
Wash T1N-R5W-S33E	0.045	0.05
Wash T1N-R5W-S33N	0.055	0.06
Wash T1N-R5W-S33W	0.045	0.05
Wash T1N-R6W-S1	0.04 - 0.06	0.04 - 0.06
Wash T1N-R6W-S11	0.06	0.055
Wash T1N-R6W-S12	0.06	0.055
Wash T1N-R6W-S17	0.02 - 0.065	0.02 - 0.065
Wash T1N-R6W-S18	0.02 - 0.065	0.02 - 0.065
Wash T1S-R2W-S32A	0.039	0.05
Wash T1S-R5W-S17	0.045 - 0.065	0.05 - 0.06
Wash T1S-R5W-S22N	0.075	0.05 - 0.075
Wash T1S-R5W-S22S	0.075	0.05 - 0.075
Wash T1S-R5W-S29	0.04 - 0.08	0.05 - 0.075
Wash T1S-R5W-S29W	0.04	0.05
Wash T1S-R6W-S05S	0.032 - 0.065	0.032 - 0.065
Wash T1S-R6W-S08	0.035 - 0.065	0.035 - 0.065
Wash T1S-R6W-S27	0.02 - 0.06	0.02 - 0.06
Wash T2N-R5W-S27N	0.06	0.05
Wash T2N-R5W-S27S	0.065	0.05
Wash T2N-R5W-S28	0.04	0.06
Wash T2N-R5W-S32	0.05	0.06
Wash T2N-R5W-S33E	0.04 - 0.06	0.06
Wash T2N-R5W-S33W	0.06	0.055

Table 5. Range of Hydraulic Roughness Coefficients (Manning's "n") (Continued)

Flooding Source	<b>Channel</b>	<u>Overbanks</u>
Wash T2N-R6W-S02	0.02 - 0.066	0.02 - 0.066
Wash T2N-R6W-S05E	0.02 - 0.075	0.02 - 0.075
Wash T2N-R6W-S05N	0.044 - 0.072	0.044 - 0.072
Wash T2N-R6W-S05S	0.032 - 0.065	0.032 - 0.065
Wash T2N-R6W-S05W	0.044 - 0.072	0.044 - 0.072
Wash T2N-R6W-S22	0.02 - 0.06	0.02 - 0.06
Wash T2N-R6W-S22S	0.035	0.035
Wash T2N-R6W-S28N	0.02 - 0.06	0.02 - 0.06
Wash T2N-R6W-S36	0.06	0.055
Wash T2N-R6W-S36W	0.045	0.05
Wash T2N-R7W-S20W	0.042 - 0.072	0.02 - 0.072
Wash T2N-R7W-S32E	0.042 - 0.065	0.02 - 0.065
Wash T2N-R7W-S35W	0.02 - 0.065	0.02 - 0.065
Wash T3N-R6W-S27W	0.044 - 0.072	0.02 - 0.072
Wash T3N-R6W-S32	0.044 - 0.072	0.044 - 0.072
Wash T3N-R6W-S33	0.044 - 0.072	0.044 - 0.072
Wash T3N-R6W-S35	0.037 - 0.066	0.037 - 0.066
Wash T5N-R4W-S3	0.040 - 0.065	0.04 - 0.065
Wash T5N-R4W-S7A	0.050	0.065
Wash T5N-R4W-S7B	0.065	0.075
Wash T5N-R4W-S7C	0.060	0.075
Wash T5N-R4W-S19	0.045	0.045 - 0.06
Wash T5N-R4W-S20A	0.040 - 0.045	0.04 - 0.11
Wash T5N-R4W-S20B	0.040 - 0.055	0.04 - 0.06
Wash T5N-R4W-S21	0.040 - 0.045	0.045 - 0.065
Wash T5N-R4W-S30	0.045	0.045 - 0.05
Wash T5N-R5W-S1	0.045	0.045 - 0.07

Table 5. Range of Hydraulic Roughness Coefficients (Manning's "n") (Continued)

Flooding Source	Channel	Overbanks
Wash T5N-R5W-S3A	0.050	0.055
Wash T5N-R5W-S3B	0.045 - 0.050	0.04 - 0.055
Wash T5N-R5W-S10A	0.050 - 0.060	0.050 - 0.055
Wash T5N-R5W-S11	0.045 - 0.055	0.045 - 0.055
Wash T5N-R5W-S12	0.050	0.065
Wash T5N-R5W-S13A	0.050	0.065
Wash T5N-R5W-S13B	0.045	0.045 - 0.065
Wash T5N-R5W-S14	0.050 - 0.055	0.05 - 0.055
Wash T5N-R5W-S14B	0.050	0.05
Wash T5N-R5W-S22	0.065	0.050 - 0.065
Wash T5N-R5W-S23A	0.050 - 0.065	0.05 - 0.06
Wash T5N-R5W-S23B	0.045 - 0.060	0.04 - 0.06
Wash T5N-R5W-S23C	0.050	0.05 - 0.055
Wash T5N-R5W-S23D	0.050	0.05 - 0.055
Wash T5N-R5W-S23E	0.050 - 0.055	0.05
Wash T5N-R5W-S23F	0.050	0.055
Wash T5N-R5W-S25A	0.045	0.045 - 0.07
Wash T5N-R5W-S25B	0.045 - 0.06	0.045 - 0.055
Wash T5N-R5W-S25C	0.040 - 0.055	0.04 - 0.065
Wash T5N-R5W-S34C	0.050 - 0.055	0.05 - 0.055
Wash T5N-R5W-S35	0.045 - 0.055	0.04 - 0.055
Wash T6-R4W-S33	0.045 - 0.065	0.045 - 0.065
Wash T6-R4W-S33A	0.040	0.04 - 0.06
Wash T6-R5W-S36	0.045	0.045 - 0.055
Wash T6-R5W-S36A	0.045	0.045 - 0.060
Wash T6-R5W-S36B	0.045	0.05 - 0.06
Waterfall Wash	0.035 - 0.050	0.070 - 0.100

Table 5. Range of Hydraulic Roughness Coefficients (Manning's "n") (Continued)

Flooding Source	<b>Channel</b>	<b>Overbanks</b>
Waterman Wash	0.016 - 0.055	0.016 - 0.065
West Fork White Peak Wash	0.024 - 0.055	0.036 - 0.060
West Garambullo Wash	0.024 - 0.055	0.036 - 0.060
West Prong of Waterman Wash	0.036 - 0.038	0.045
White Granite Wash	0.035	0.070
White Peak Wash	0.024 - 0.055	0.036 - 0.060
White Spar Wash	0.025 - 0.06	0.025 - 0.5
White Tanks Wash No. 3	0.035 - 0.045	0.035 - 0.070
Winters Wash	0.02 - 0.065	0.02 - 0.065
Wittmann Area Washes	0.015 - 0.060	0.015 - 0.090
Wittmann Wash Tributary 1	0.033 - 0.077	0.033 - 0.077
Wittmann Wash Tributary 1 Breakout 1	0.033-0.059	0.033-0.059
Wittmann Wash Tributary 1 Breakout 1 of Breakout 3	0.035-0.069	0.035-0.069
Wittmann Wash Tributary 1 Breakout 2	0.033-0.059	0.033-0.059
Wittmann Wash Tributary 1 Breakout 3	0.035-0.069	0.035-0.069
Wittmann Wash Tributary 1 Breakout 4	0.035-0.069	0.035-0.069
Willow Springs Wash	0.020 - 0.045	0.020 - 0.080
Willow Springs Wash Tributary 1	0.030 - 0.040	0.035 - 0.055
Willow Springs Wash Tributary 1A	0.028 - 0.050	0.040 - 0.060
Willow Springs Wash Tributary 2	0.030 - 0.055	0.045 - 0.060
Willow Springs Wash Tributary 2A	0.040 - 0.050	0.050 - 0.055
Willow Springs Wash Tributary 3	0.060	0.080
Willow Springs Wash Tributary 4	0.040 - 0.050	0.050
Willow Springs Wash Tributary 5	0.035 - 0.050	0.045 - 0.060
Willow Springs Wash Tributary 5A	0.040	0.045 - 0.050

Salt River photos for the 1978 and 1980 flooding events were extensively used in establishing channel parameters for bank station identification, "n" values, and flood flow

conveyance patterns. Information from the current airport channelization project was also transferred to the maps. The Salt River model also includes the proposed south dike on the Salt River, which represents an extension of the airport channelization project. This dike is located between Hohokam Expressway (48th Street) and Priest Road on the southern bank of the Salt River.

Water-surface elevations computed in the HEC-2 hydraulic model were calibrated with the known floodplains of the 1978 and 1980 flooding events. This technique involved the adjustment at conveyance boundaries and "n" values.

The starting water-surface elevation for Scatter Wash was taken from Skunk Creek. Manning's "n" values were determined through field investigations and engineering judgment. Scatter Wash is a relatively flat floodplain for the majority of its reach, with a substantial amount of development in some over bank areas. In the upper Scatter Wash drainage basin, it was determined that flood flows would proceed along the many braided streamlines, until they reach I-17. At I-17, the flows will begin to concentrate in the area north of Williams Road. The 1-percent-annual-chance flows at this point will separate into a north and south branch of Scatter Wash. The Scatter Wash, North Branch, passes under I-17 through two culverts, and over I-17 via Sheet flow action. Scatter Wash, South Branch, continues to flow southerly along the eastern side of I-17, until it eventually ponds and passes under I-17 at Deer Valley Road. Both branches of Scatter Wash join in the vicinity of Rose Garden Lane and 33rd Avenue. At this location, the flows proceed downstream to their confluence with Skunk Creek.

During periods of heavy runoff, flows from Sand Tank and Bender Washes near Gila Bend are intermixed. Highway and railroad bridges traverse both washes. These structures cannot pass a 1-percent-annual-chance flood, resulting in extensive ponding at each obstruction during floods of low frequency.

Apache Creek is located on an alluvial fan near Apache Junction at the base of the Superstition Mountains. A vast network of intermingling channels exists on the fan. Flooding on alluvial fans is often erratic and unpredictable, and flow may occur on separate parts of an alluvial fan during sequent flood events. Flooding in this area was analyzed using alluvial fan methodology by FEMA.

Much of the flooding in the county is caused by sheet flow that originates from alluvial fans. Flows are intercepted by canal levees, railroad embankments, and elevated roads, causing water to pond behind the embankments. Depths of ponding depend on the elevation of the embankments. When the intercepted runoff exceeds ponding storage capacity, the flow will overtop the embankment, thus eroding the levee. Areas immediately down slope of the breakout will be affected by high water. However, flows will fan out to again become shallow sheet flow that is less than 1 foot in depth. Therefore, many areas in the county have been designated Zone X.

Approximate hydraulic analyses for Bulldog, Apache, and Goldfield Washes and the downstream reach of Weekes Wash were carried out using approximate flow velocities and normal-depth calculations. These analyses revealed that the channels have very little capacity relative to the 1-percent-annual-chance flood, and in some cases, the channels

are nonexistent. Furthermore, the overbank flow is not confined to a well-defined floodplain, causing shallow flooding. The average depth of flooding for the over bank areas was determined to be less than 1 foot.

Areas of ponding on the upstream side of U.S. Highway 60/89 were also studied. Water-surface elevations for these areas were based on the elevation of the highway grade with shallow flows over the highway of less than 1 foot. This results in average shallow flooding depths behind the highway between 1 and 3 feet.

Cross sections were taken perpendicular to the canals and railroad embankments using topographic maps (Aerial Mapping Company, 1977). The top of the embankments were assumed to be the maximum ponding elevation up slope of the embankment. Flood hazard areas were then determined by projecting this elevation up slope to intersect the natural ground.

The canal levees and railroad embankments do not permanently retain storm flows, but divert them along the embankments. Most of the canal levees consist of unconsolidated material. These levees are subject to failure when runoff volumes exceed storage capacity. Potential flood hazard areas on the down slope side of the canals were analyzed for levees exceeding 2 feet in height. This analysis determined the distance required for flow through a break in a levee to spread and be reduced to an average depth of 1 foot, using Manning's equation. This analysis assumed the following:

- 1. A canal breach could occur at any point.
- 2. A broad, cresting horizontal weir equation with a head of three feet could be used to determine the length of a breach, resulting in a weir from 50 to 100 feet long.
- 3. Floodwaters would spread at a 45 degree angle from the breach in the levee.
- 4. The peak discharge at a potential levee break was the maximum canal capacity or the concentration of peak flows from runoff in the watershed, whichever was greater.

Due to the nature of flooding along the New River, Skunk Creek below Carefree Highway, Lower El Mirage Wash, Scatter Wash below Black Canyon Highway and East Branch Scatter Wash, no 0.2-percent-annual-chance flood profiles were developed. The floodplains of these streams are wide; therefore, flow could increase substantially without significantly raising the water-surface elevation or increasing the velocity of flow. Moreover, most of the area contiguous to the floodplains is subject to sheet flow during a 1-percent-annual-chance flood.

In addition, 2-percent-annual-chance flood profiles for the Agua Fria and New Rivers, Skunk Creek below Carefree Highway, Cave Creek below Cave Creek Dam, East Fork of Cave Creek, and Echo Canyon, Scatter, and East Branch Scatter Washes were not computed.

Flood profiles are not applicable for areas of shallow flooding and ponding; therefore, flood profiles are not presented for any of the canals or other areas of shallow flooding, including Sand Tank and Bender Washes, Rodeo Wash and its tributary, Lower El Mirage Wash Tributary, and Airport and Scott Avenue Washes.

For flooding sources studied by approximate methods, 1-percent-annual-chance flood elevations were computed using Manning's equation, USACE Floodplain Information reports (U.S. Department of the Army, 1967; U.S. Department of the Army, 1972; U.S. Department of the Army, 1965; U.S. Department of the Army, 1964), USGS Flood-Prone Area Maps (U.S. Department of the Interior, 1969, 1972), USGS slope maps (U.S. Department of the Interior, 1974-75), high-resolution Skylab photographs (National Aeronautics and Space Administration, Skylab Earth Terrain Camera Color Photography, 1973; National Aeronautics and Space Administration, Skylab Multispectral Camera Black and White Photography), and USGS topographic maps (U.S. Department of the Interior, 1964, et cetera).

The study was limited to the uses of fixed-bed modeling for the hydraulic analyses. However, with the occurrence of a large flood, substantial changes in the riverbed are expected to occur, particularly where the bottom slope is very non-uniform and/or where other structures, such as bridges, cause local increases in the velocity. Resultant changes in the water-surface elevations can be expected.

The hydraulic analyses for the levee failure scenario for Stagecoach Pass Wash were based on the HEC-RAS model developed by DEI Professional Services (2005). For the levee failure scenario the levee was removed from the cross section and the resulting water-surface elevations determined.

Cross-section data for the Stagecoach Pass Wash Overflow hydraulic analyses were obtained from digital topographic mapping developed from Grading Plan Sand Flower II (American Engineering Company, 1954) and North Scottsdale Floodplain Delineation Study (DEI Professional Services, 2006). The starting water-surface elevation was determined by the slope-area method. Water-surface elevations were computed using the USACE HEC-RAS step-backwater computer program (USACE, 2002).

The hydraulic analyses for this study were based on unobstructed flow. The flood elevations shown on the profiles are thus considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail.

To obtain current elevation, description, and/or location information for National Geodetic Survey bench marks shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242, or visit its website at http://www.ngs.noaa.gov. To obtain information about Geodetic Densification and Cadastral Survey bench marks produced by the Maricopa County Department of Transportation, please visit the Flood Control District of Maricopa County website at http://www.fcd.maricopa.gov/Maps/gismaps/apps/gdacs/application/index.cfm.

## 3.3 Vertical Datum

All FISs and FIRMs are referenced to a specific vertical datum. The vertical datum provides a starting point against which flood, ground, and structure elevations can be referenced and compared. Until recently, the standard vertical datum in use for newly created or revised FISs and FIRMs was the National Geodetic Vertical Datum of 1929 (NGVD 29). With the finalization of the North American Vertical Datum of 1988 (NAVD 88), many FIS reports and FIRMs are being prepared using NAVD 88 as the referenced vertical datum.

All flood elevations shown in this FIS report and on the FIRM are referenced to NAVD 88. Structure and ground elevations in the community must, therefore, be referenced to NAVD 88. It is important to note that adjacent communities may be referenced to NGVD 29. This may result in differences in base flood elevations across the corporate limits between the communities.

Prior versions of the FIS report and FIRM were referenced to NGVD 29. When a datum conversion is effected for an FIS report and FIRM, the Flood Profiles, base flood elevations (BFEs) and ERMs reflect the new datum values. To compare structure and ground elevations to 1-percent-annual-chance flood elevations shown in the FIS and on the FIRM, the subject structure and ground elevations must be referenced to the new datum values.

As noted above, the elevations shown in the FIS report and on the FIRM for Maricopa County are referenced to NAVD 88. Ground, structure, and flood elevations may be compared and/or referenced to NGVD 29 by applying a standard conversion factor.

Due to the statistically significant range in conversion factors, an average conversion factor could not be established for the entire community. The elevations shown in the FIS report and on the FIRM were, therefore, converted to NAVD 88 using information provided by Maricopa County. Users wishing to obtain flood elevations referenced to NGVD 29 may use the following Maricopa County website application: http://www.fcd.maricopa.gov/Maps/gismaps/apps/gdacs/application/index.cfm. This web tool allows users to obtain point-specific datum conversion values by zooming in and hovering over a Vertcon point with their mouse. The application requires that the Vertcon layer be turned on by selecting the Vertcon checkbox on the layers menu on the left side of the screen. The Vertcon grid referenced in this web application was also used to convert existing flood elevations from NGVD 29 to NAVD 88.

The BFEs shown on the FIRM represent whole-foot rounded values. For example, a BFE of 102.4 will appear as 102 on the FIRM and 102.6 will appear as 103. Therefore, users that wish to convert the elevations in this FIS to NGVD 29 should apply the stated conversion factor(s) to elevations shown on the Flood Profiles and supporting data tables in the FIS report, which are shown at a minimum to the nearest 0.1 foot.

For more information on NAVD 88, see <u>Converting the National Flood Insurance Program to the North American Vertical Datum of 1988</u>, FEMA Publication FIA-20/June 1992.

## 4.0 FLOODPLAIN MANAGEMENT APPLICATIONS

The NFIP encourages State and local governments to adopt sound floodplain management programs. To assist in this endeavor, each FIS provides 1-percent-annual-chance floodplain data, which may include a combination of the following: 10-, 2-, 1-, and 0.2-percent-annual-chance flood elevations; delineations of the 1 and 0.2-percent-annual-chance floodplains; and 1-percent-annual-chance floodway. This information is presented on the FIRM and in many components of the FIS, including Flood Profiles, Floodway Data tables, and Summary of Stillwater Elevation tables. Users should reference the data presented in the FIS as well as additional information that may be available at the local community map repository before making flood elevation and/or floodplain boundary determinations.

## 4.1 Flood Boundaries

To provide a national standard without regional discrimination, the 1-percent-annual-chance flood has been adopted by FEMA as the base flood for floodplain management purposes. The 0.2-percent-annual-chance flood is employed to indicate additional areas of flood risk in the community. For each stream studied by detailed methods, the 1- and 0.2-percent-annual-chance floodplain boundaries have been delineated using the flood elevations determined at each cross section. Between cross sections, the boundaries were interpolated using topographic maps at scales of 1:1,200, 1:2,400, 1:4,800, and 1:6,000, with contour intervals of 2 and 4 feet (Harris-Toups Associates, 1976; City of Phoenix, 1967; U.S. Department of the Army, 1976; and Aerial Mapping Company, 1977).

The 1- and 0.2-percent-annual-chance floodplain boundaries are shown on the FIRM (Exhibit 1). In cases where the 1- and 0.2-percent-annual-chance floodplain boundaries are close together, only the 1-percent-annual-chance floodplain boundary has been shown. Small areas within the floodplain boundaries may lie above the flood elevations but cannot be shown due to limitations of the map scale and/or lack of detailed topographic data.

Approximate flood boundaries were delineated using USGS topographic maps and Flood-Prone Areas Maps (U.S. Department of the Interior, 1969, 1972; U.S. Department of the Interior, 1964, et cetera), and high-resolution Skylab photographs (National Aeronautics and Space Administration, Skylab Earth Terrain Camera Color Photography, 1973; National Aeronautics and Space Administration, Skylab Multispectral Camera Black and White Photography).

For Stagecoach Pass Wash the 1- and 0.2-percent-annual-chance floodplain boundaries have been delineated using the flood elevations determined at each cross section. Between cross sections, the boundaries were interpolated using digital topographic mapping developed from Grading Plan Sand Flower II (American Engineering Company, 1954) and North Scottsdale Floodplain Delineation Study (DEI Professional Services, 2006).

For Stagecoach Pass Wash Overflow the 1-percent-annual-chance floodplain boundaries have been delineated using the flood elevations determined at each cross section. Between cross sections, the boundaries were interpolated using digital topographic

mapping developed from Grading Plan Sand Flower II (American Engineering Company, 1954) and North Scottsdale Floodplain Delineation Study (DEI Professional Services, 2006).

## 4.2 Floodways

Encroachment on floodplains, such as structures and fill, reduces flood-carrying capacity, increases flood heights and velocities, and increases flood hazards in areas beyond the encroachment itself. One aspect of floodplain management involves balancing the economic gain from floodplain development against the resulting increase in flood hazard. For purposes of the NFIP, a floodway is used as a tool to assist local communities in this aspect of floodplain management. Under this concept, the area of the 1-percent-annual-chance floodplain is divided into a floodway and a floodway fringe. The floodway is the channel of a stream, plus any adjacent floodplain areas, that must be kept free of encroachment so that the 1-percent-annual-chance flood can be carried without substantial increases in flood heights. Minimum Federal standards limit such increases to 1.0 foot, provided that hazardous velocities are not produced. The floodways in this study are presented to local agencies as minimum standards that can be adopted directly or that can be used as a basis for additional floodway studies.

The floodways presented in this study were computed on the basis of equal-conveyance reduction from each side of the floodplain. The results of these computations are tabulated at selected cross sections for each stream segment for which a floodway is computed (Table 6).

As shown on the FIRM (Exhibit 1), the floodway boundaries were computed at cross sections. Between cross sections, the boundaries were interpolated. In cases where the floodway and 1-percent-annual-chance floodplain boundaries are either close together or collinear, only the floodway boundary has been shown.

The floodways for Little San Domingo, Mockingbird, and Powder House Washes are shown coincident with the 1-percent-annual-chance floodplain boundaries because of high, hazardous velocities in their respective floodplains. No floodway was computed for Cave Creek below Arizona Canal. No floodway was computed for Wittmann Drainage due to the split flow below Center Street. Floodways for Grass Wash below the U.S. Highway 60 bridge and for Aguila Farm Channel were not computed due to excessive overbank losses.

Floodways are not applicable for areas of shallow flooding; therefore, floodways were not computed for any of the canals, railroad embankments, or for Sand Tank and Bender Washes, Rodeo Wash and its tributary, Airport and Scott Avenue Washes, Lower El Mirage Wash Tributary, and Apache Creek.

Administrative floodways have been established for some approximate SFHAs to aid local officials in regulating and managing of these areas.

The area between the floodway and 1-percent-annual-chance floodplain boundaries is termed the floodway fringe. The floodway fringe encompasses the portion of the

floodplain that could be completely obstructed without increasing the water-surface elevation of the 1-percent-annual-chance flood by more than 1.0 foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to floodplain development are shown in Figure 6.

- LIMIT OF FLOODPLAIN FOR UNENCROACHED 1% ANNUAL CHANCE FLOOD-FLOODWAY FLOODWAY FLOODWAY-FRINGE FRINGE STREAM CHANNEL FLOOD ELEVATION WHEN CONFINED WITHIN FLOODWAY GROUND SURFACE ENCROACHMENT **ENCROACHMENT** С SURCHARGE\* AREA OF ALLOWABLE ENCROACHMENT; RAISING GROUND SURFACE WILL FILL FLOOD ELEVATION BEFORE ENCROACHMENT ON FLOODPLAIN NOT CAUSE A SURCHARGE THAT EXCEEDS THE INDICATED STANDARDS LINE A - B IS THE FLOOD ELEVATION BEFORE ENCROACHMENT LINE C - D IS THE FLOOD ELEVATION AFTER ENCROACHMENT

\*SURCHARGE NOT TO EXCEED 1.0 FOOT (FEMA REQUIREMENT) OR LESSER HEIGHT IF SPECIFIED BY STATE OR COMMUNITY.

Figure 6. Floodway Schematic

FLOODING	FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)		
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Amir Wash								
A B C D E F G H I J K L M N O P Q	2,190 2,216 2,790 3,301 3,752 4,584 5,816 6,950 7,898 8,471 9,837 10,727 11,720 12,687 13,816 14,722 15,170	60 395 77 37 30 137 72 135 70 61 56 72 121 42 90 80 46	331 1,816 334 153 150 498 223 263 215 196 166 141 177 113 153 144 98	5.9 1.0 5.5 11.9 12.2 3.7 8.2 6.9 8.5 9.3 6.3 7.4 5.9 9.2 5.3 5.6 8.2	2,092.7 2,093.3 2,094.3 2,102.5 2,109.2 2,123.4 2,142.4 2,158.3 2,173.8 2,181.7 2,200.4 2,211.5 2,225.4 2,238.0 2,251.0 2,262.8 2,268.3	2,092.7 2,093.3 2,094.3 2,102.5 2,109.2 2,123.4 2,142.4 2,158.3 2,173.8 2,181.7 2,200.4 2,211.5 2,225.4 2,238.0 2,251.0 2,262.8 2,268.3	2,092.7 2,093.3 2,094.3 2,102.5 2,109.4 2,123.7 2,143.2 2,159.2 2,174.5 2,182.7 2,201.2 2,211.5 2,225.4 2,238.4 2,251.8 2,263.5 2,268.8	0.0 0.0 0.0 0.0 0.2 0.3 0.8 0.9 0.7 1.0 0.8 0.0 0.0 0.4 0.8 0.7 0.5

<sup>&</sup>lt;sup>1</sup>Feet above confluence with Hassayampa River

↓	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
BLE	MARICOPA COUNTY,AZ	. 200211111 211111
<u>ი</u>	AND INCORPORATED AREAS	AMIR WASH

FLOODING SOURCE FLOODWAY BASE FLOOD WATER (FEET			LOOD WATER S (FEET NA	-	TION			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Blue Tank Wash								
A B	1,088 1,771	225 277	629 660	7.8 7.4	2,077.5 2,091.8	2,077.5 2,091.8	2,077.9 2,092.3	0.4 0.5
C D	2,162 2,517	240 180	636 577	7.7 8.5	2,101.1 2,109.5	2,101.1 2,109.5	2,101.9 2,110.5	0.8 1.0
E F	3,103 3,779	150 150	553 575	8.9 8.5	2,123.5 2,139.5	2,123.5 2,139.5	2,124.2 2,140.3	0.7 0.8
G H	4,642 5,525	180 226	558 652	8.8 7.5	2,156.5 2,175.4	2,156.5 2,175.4	2,157.0 2,176.4	0.5 1.0
"	0,020	220	002	7.0	2,170.4	2,170.7	2,170.7	1.0

<sup>&</sup>lt;sup>1</sup>Feet above the confluence with Hassayampa River

1/	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
BLE	MARICOPA COUNTY,AZ	120051171
6	AND INCORPORATED AREAS	BLUE TANK WASH

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/ SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Calamity Wash								
A B C D E	259 917 1,443 1,912 2,439	136 43 40 36 66	416 257 252 243 305	10.3 13.8 14.1 14.6 11.6	2,027.7 2,043.8 2,056.5 2,069.9 2,081.4	2,027.7 2,043.8 2,056.5 2,069.9 2,081.4	2,027.7 2,043.8 2,056.5 2,070.0 2,018.5	0.0 0.0 0.0 0.1 0.1
G	2,936 3,367	144 61	410 289	8.7 12.3	2,094.6 2,102.1	2,094.6 2,102.1	2,094.6 2,102.1	0.0 0.0
H I J K L M N O P	3,878 4,321 4,748 5,406 5,897 6,413 6,913 7,337 7,873	44 46 75 79 35 45 75 90	260 264 320 313 236 261 331 323 387	13.7 13.5 10.9 11.2 14.8 13.4 10.6 10.8 9.0	2,116.9 2,130.2 2,140.3 2,155.8 2,171.6 2,182.2 2,192.3 2,202.4	2,116.9 2,130.2 2,140.3 2,155.8 2,171.6 2,182.2 2,192.3 2,202.4 2,213.0	2,116.9 2,130.2 2,140.8 2,155.8 2,171.6 2,182.6 2,192.9 2,202.9 2,213.9	0.0 0.0 0.5 0.0 0.0 0.4 0.6 0.5
Q	8,387	130	356	9.0 8.9	2,213.0 2,225.2	2,213.0 2,225.2	2,213.9	0.9
R S T	8,387 8,888 9,378 9,865	135 84 47	368 321 247	8.9 8.7 9.9 12.9	2,223.2 2,236.9 2,249.7 2,263.1	2,223.2 2,236.9 2,249.7 2,263.1	2,225.6 2,237.9 2,250.7 2,263.1	1.0 1.0 0.0

<sup>1</sup>Feet above confluence with Hassayampa River

٦A	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
BLE	MARICOPA COUNTY, AZ	
6	AND INCORPORATED AREAS	CALAMITY WASH

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Calamity Wash (continued)								
U V W	10,318 10,852 11,343	25 40 31	199 234 216	16.0 13.6 14.7	2,277.1 2,312.1 2,330.7	2,277.1 2,312.1 2,330.7	2,277.1 2,312.1 2,331.0	0.0 0.1 0.3

<sup>1</sup>Feet above confluence with Hassayampa River

17/	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA		
BLE	MARICOPA COUNTY, AZ	TEOODWAT DATA		
6	AND INCORPORATED AREAS	CALAMITY WASH		

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Casandro Wash								
А	711	22	76	4.3	2,067.8	2,067.8	2,067.8	0.0
В	1,050	41	44	4.0	2,069.3	2,069.3	2,069.4	0.1
С	1,592	34	38	5.8	2,076.1	2,076.1	2,076.2	0.1
D	2,178	36	32	3.9	2,082.5	2,082.5	2,082.6	0.1
E	2,593	33	52	6.4	2,089.8	2,089.8	2,089.8	0.0
F	3,035	46	61	5.1	2,094.7	2,094.7	2,094.7	0.0
G	3,535	58	160	1.8	2,105.6	2,105.6	2,106.4	0.8
Н	4,115	29	45	7.2	2,113.2	2,113.2	2,113.2	0.0
I	4,583	31	9	3.1	2,121.2	2,121.2	2,121.2	0.0
J	5,148	86	14	2.0	2,132.5	2,132.5	2,132.5	0.0
K	5,765	411	287	5.7	2,155.2	2,143.8 <sup>2</sup>	2,143.8	0.0
L	6,866	43	155	10.6	2,168.3	2,168.3	2,168.3	0.0
M	7,764	43	157	10.4	2,185.1	2,185.1	2,185.5	0.4
N	9,011	48	172	9.5	2,201.1	2,201.1	2,201.4	0.3
0	9,603	76	188	8.7	2,209.9	2,209.9	2,210.6	0.7
Р	10,675	49	133	8.1	2,222.3	2,222.3	2,222.4	0.1
Q	11,718	27	104	10.4	2,236.1	2,236.1	2,236.1	0.0
R	12,765	35	172	6.3	2,253.0	2,253.0	2,253.3	0.3
S	13,750	39	112	9.6	2,262.3	2,262.3	2,262.6	0.3
Т	14,661	88	101	5.7	2,276.9	2,276.9	2,276.9	0.0

17	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA			
BLE	MARICOPA COUNTY, AZ				
6	AND INCORPORATED AREAS	CASANDRO WASH			

<sup>&</sup>lt;sup>1</sup>Feet above confluence with Sols Wash <sup>2</sup>Elevation computed without consideration of backwater effects from Casandro Wash Dam

FLOODING	FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Casandro Wash South Branch									
A B C D E F	238 1,005 2,107 3,245 4,262 5,058	24 35 47 24 18 16	83 87 117 88 71 43	7.0 8.7 5.3 5.6 5.7 9.3	2,220.0 2,233.2 2,247.8 2,265.5 2,281.5 2,292.0	2,220.0 2,233.2 2,247.8 2,265.5 2,281.5 2,292.0	2,220.8 2,233.2 2,248.3 2,265.7 2,281.9 2,292.0	0.8 0.0 0.5 0.2 0.4 0.0	

<sup>1</sup>Feet above confluence with Casandro Wash

1,T	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
BLE	MARICOPA COUNTY, AZ	
6	AND INCORPORATED AREAS	CASANDRO WASH SOUTH BRANCH

FLOODING SOURCE			FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Cemetery Wash									
A B	1,045 1,483	233 281	875 1,407	9.1 5.7	2,029.9 2,040.6	2,029.9 2,040.6	2,030.7 2,040.7	0.8 0.1	
C	2,230	461	3,109	6.1	2,048.9	2,048.9	2,049.2	0.3	
D	2,584	286	1,264	15.3	2,052.9	2,052.9	2,052.9	0.0	
E	3,020	270	1,053	7.6	2,058.8	2,058.8	2,059.6	0.8	
F	3,542	163	806	9.9	2,067.3	2,067.3	2,067.9	0.6	
G	4,142	110	665	12.0	2,079.1	2,079.1	2,079.5	0.4	
Н	4,564	65	529	15.1	2,086.6	2,086.6	2,087.2	0.6	
I	5,020	49	460	17.4	2,096.3	2,096.3	2,096.4	0.1	
J	5,410	115	788	10.1	2,101.5	2,101.5	2,101.9	0.4	
K	6,049	80	546	14.6	2,111.5	2,111.5	2,111.5	0.0	
L	6,312	98	602	13.3	2,118.8	2,118.8	2,118.9	0.1	
M	7,197	171	803	9.9	2,129.2	2,129.2	2,130.1	0.9	
N	7,773	130	711	11.2	2,137.7	2,137.7	2,138.5	0.8	
0	8,118	145	722	11.0	2,141.7	2,141.7	2,142.6	0.9	
Р	8,613	123	666	12.0	2,150.4	2,150.4	2,151.0	0.6	
Q	9,141	95	617	12.9	2,158.3	2,158.3	2,158.9	0.6	
R	9,355	102	592	13.5	2,160.7	2,160.7	2,161.5	0.8	
S	10,007	275	969	7.8	2,169.7	2,169.7	2,170.6	0.9	
Т	10,696	265	854	8.9	2,181.4	2,181.4	2,182.3	0.9	
U	11,008	243	815	9.3	2,186.4	2,186.4	2,187.3	0.9	
V	11,464	140	713	10.7	2,194.4	2,194.4	2,195.3	0.9	

<sup>&</sup>lt;sup>1</sup>Feet above confluence with Hassayampa River

1,T	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
BLE	MARICOPA COUNTY, AZ	. 200211711
6	AND INCORPORATED AREAS	CEMETERY WASH

FLOODING SOURCE			FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Cemetery Wash (continued)				,					
W	12,033	147	717	10.6	2,203.2	2,203.2	2,204.0	0.8	
X	12,567	92	555	13.7	2,211.0	2,211.0	2,211.8	0.8	
Y	13,255	111	584	13.0	2,223.3	2,223.3	2,223.3	0.1	
z	14,496	100	566	13.4	2,244.1	2,244.1	2,244.5	0.4	
AA	15,430	95	586	11.8	2,260.1	2,260.1	2,260.8	0.7	
AB	16,457	65	457	15.1	2,274.6	2,274.6	2,275.0	0.4	
AC	17,462	122	651	10.6	2,289.2	2,289.2	2,290.1	0.9	
AD	18,891	114	393	10.3	2,310.3	2,310.3	2,311.0	0.7	
AE	19,421	85	367	11.0	2,319.0	2,319.0	2,319.7	0.7	
AF	20,347	159	487	8.3	2,334.2	2,334.2	2,335.1	0.9	
AG	21,359	156	459	8.8	2,352.2	2,352.2	2,353.1	0.9	
AH	22,564	148	455	8.9	2,373.1	2,373.1	2,373.7	0.6	
Al	23,418	197	530	7.5	2,385.7	2,385.7	2,386.7	1.0	
AJ	24,391	155	441	9.0	2,402.2	2,402.2	2,403.0	0.8	
AK	25,411	130	477	8.3	2,418.2	2,418.2	2,419.0	0.8	
AL	26,577	284	650	6.1	2,436.7	2,436.7	2,437.6	0.9	
AM	27,449	178	518	7.7	2,449.1	2,449.1	2,449.9	0.8	
AN	28,223	118	294	7.4	2,461.0	2,461.0	2,461.2	0.2	
AO	29,463	116	303	7.2	2,481.8	2,481.8	2,482.8	1.0	
AP	30,425	141	334	6.5	2,498.3	2,498.3	2,499.3	1.0	
AQ	31,401	65	219	9.9	2,513.7	2,513.7	2,514.6	0.9	
AR	32,033	60	224	9.7	2,522.6	2,522.6	2,523.4	0.8	

<sup>&</sup>lt;sup>1</sup>Feet above confluence with Hassayampa River

1	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
BLE	MARICOPA COUNTY, AZ	
6	AND INCORPORATED AREAS	CEMETERY WASH

FLOODING	FLOODING SOURCE		FLOODWAY		BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			TION
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Cemetery Wash								
Tributary R-1								
A	190	67	450	2.2	2,253.5	2,253.5	2,254.0	0.5
В	1,056	53	117	8.4	2,263.8	2,263.8	2,263.8	0.0
С	2,087	20	84	11.7	2,280.5	2,280.5	2,280.5	0.0
D	3,124	37	107	9.2	2,301.6	2,301.6	2,301.7	0.1
E	4,185	62	127	7.7	2,325.8	2,325.8	2,325.8	0.0
F	5,103	40	110	8.9	2,340.9	2,340.9	2,341.0	0.1
G	6,005	44	115	7.5	2,355.7	2,355.7	2,355.8	0.1
Н	7,136	51	139	6.2	2,373.5	2,373.5	2,373.5	0.0
I	8,375	52	114	7.6	2,393.6	2,393.6	2,393.7	0.1
J	9,292	38	104	8.4	2,411.3	2,411.3	2,411.3	0.0
K	10,193	22	31	6.8	2,428.0	2,428.0	2,428.0	0.0
L	11,120	21	31	7.0	2,449.3	2,449.3	2,449.3	0.0
M	12,019	30	35	6.0	2,475.1	2,475.1	2,475.1	0.0
N	12,989	24	32	6.6	2,499.6	2,499.6	2,499.6	0.0

<sup>&</sup>lt;sup>1</sup>Feet above confluence with Cemetery Wash

1	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
BLE	MARICOPA COUNTY,AZ	
6	AND INCORPORATED AREAS	CEMETERY WASH TRIBUTARY R-1

FLOODING	SOURCE		FLOODWAY			LOOD WATER S (FEET NA	URFACE ELEVA VD88)	TION
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Cemetery Wash Tributary R-2								
Α	797	139	432	7.1	2,308.6	2,308.6	2,309.0	0.4
В	1,859	144	443	6.9	2,325.9	2,325.9	2,325.9	0.0
С	2,705	221	531	5.8	2,339.7	2,339.7	2,340.6	0.9
D	3,799	198	452	5.5	2,356.2	2,356.2	2,357.1	0.9
Е	4,787	158	484	5.1	2,372.0	2,372.0	2,372.0	0.0
F	5,796	141	321	7.7	2,387.0	2,387.0	2,387.8	0.8
G	6,589	90	251	9.3	2,401.4	2,401.4	2,401.7	0.3
Н	7,620	103	252	9.2	2,419.1	2,419.1	2,419.2	0.1
I	8,616	310	390	6.0	2,433.5	2,433.5	2,433.9	0.4
J	9,513	217	441	5.3	2,448.1	2,448.1	2,449.0	0.9
K	10,500	195	353	6.3	2,462.0	2,462.0	2,462.9	0.9
L	11,485	110	278	8.0	2,478.4	2,478.4	2,478.4	0.0
M	12,430	190	378	5.9	2,490.4	2,490.4	2,491.1	0.7
N	13,428	225	307	5.6	2,502.8	2,502.8	2,503.6	0.8
0	14,641	240	300	5.7	2,519.5	2,519.5	2,520.2	0.7

<sup>1</sup>Feet above confluence with Cemetery Wash

1,	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
BLE	MARICOPA COUNTY, AZ	1 2005117(1 57(17)
6	AND INCORPORATED AREAS	CEMETERY WASH TRIBUTARY R-2

FLOODING	FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Cemetery Wash Tributary R-3									
A B C D E Cemetery Wash Tributary R-4	607 1,537 2,696 3,686 4,507	138 168 67 74 30	272 302 196 133 90	6.4 5.8 8.9 6.6 9.8	2,458.8 2,474.8 2,494.4 2,509.7 2,522.4	2,458.8 2,474.8 2,494.4 2,509.7 2,522.4	2,459.6 2,475.8 2,494.9 2,510.5 2,523.1	0.8 1.0 0.5 0.8 0.7	
A	750	60	148	7.2	2,535.3	2,535.3	2,536.3	1.0	

<sup>&</sup>lt;sup>1</sup>Feet above confluence with Cemetery Wash

7	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA				
₽BL	MARICOPA COUNTY, AZ	I LOODWAT DATA				
ш	MINITED IN 3001111, NE	CEMETERY WASH TRIBUTARY R-3 – CEMETERY WASH				
တ	AND INCORPORATED AREAS	TRIBUTARY R-4				

FLOODING SOURCE			FLOODWAY	,	BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/ SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Centennial Wash								
Α	6,238	2,780	11,721	3.8	778.2	778.2	779.2	1.0
В	7,305	1,679	9,783	4.7	784.1	784.1	784.9	0.8
С	8,223	1,485	9,832	5.3	786.5	786.5	787.4	0.9
D	9,331	1,792	10,766	4.1	789.1	789.1	789.7	0.6
Е	10,373	1,934	10,543	4.2	792.1	792.1	792.7	0.6
F	11,607	2,602	18,738	2.4	796.1	796.1	796.9	0.8
G	12,307	2,737	17,960	2.5	797.1	797.1	798.0	0.9
Н	13,241	2,883	16,880	2.6	799.5	799.5	800.2	0.7
1	14,438	2,726	18,798	2.3	802.9	802.9	803.7	0.8
J	15,322	2,558	18,717	2.4	804.9	804.9	805.8	0.9
K	16,264	2,506	19,020	2.3	806.7	806.7	806.6	0.9
L	17,269	3,072	22,761	1.9	808.3	808.3	809.1	0.9
M	18,231	3,239	19,617	2.3	809.0	809.0	810.0	1.0
N	19,327	2,854	13,638	3.2	810.9	810.9	811.6	0.7
0	20,290	2,364	12,763	3.5	814.3	814.3	814.8	0.5
Р	21,494	2,018	14,151	3.1	816.8	816.8	817.3	0.6
Q	22,292	1,884	11,737	3.8	817.8	817.8	818.5	0.7
R	23,567	2,109	13,879	3.2	820.3	820.3	820.9	0.6
S	24,298	2,439	12,524	3.5	821.5	821.5	822.2	0.7

<sup>1</sup>Feet above confluence with Gila River

FEDERAL EMERGENCY MANAGEMENT AGENCY

MARICOPA COUNTY, AZ

AND INCORPORATED AREAS

FLOODWAY DATA

CENTENNIAL WASH

FLOODING	FLOODING SOURCE		FLOODWAY		BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Centennial Wash (continued)								
Т	25,508	2,796	13,777	3.2	823.7	823.7	824.7	1.0
U	26,349	2,770	13,146	3.4	825.2	825.2	826.2	0.9
V	27,423	2,673	13,383	3.3	828.0	828.0	828.9	0.9
W	28,172	2,735	13,369	3.3	829.9	829.9	830.7	0.7
X	29,425	2,778	12,967	3.4	832.4	832.4	833.3	0.9
Υ	30,303	2,960	13,997	3.2	834.1	834.1	834.9	0.8
Z	31,201	3,446	14,858	3.0	835.7	835.7	836.3	0.6
AA	32,392	4,722	16,378	3.0	838.2	838.2	839.0	0.8
AB	33,573	5,879	21,611	2.4	840.8	840.8	841.6	0.8
AC	34,384	6,210	23,546	2.2	841.9	841.9	842.6	0.7
AD	35,205	10,394	3,779	5.7	848.4	848.4	849.4	1.0
AE	37,193	10,361	4,199	5.7	854.5	854.5	855.0	0.5
AF	39,000	10,339	5,025	4.7	859.3	859.3	859.8	0.5
AG	41,345	10,464	17,671	1.6	859.6	859.6	860.5	0.9
AH	43,330	3,444	14,314	3.1	861.8	861.8	862.1	0.3
Al	45,329	3,271	11,795	3.3	865.3	865.3	865.9	0.6
AJ	47,328	4,744	14,149	2.7	869.0	869.0	869.5	0.5
AK	49,343	3,728	10,407	3.7	873.3	873.3	873.3	0.1
AL	51,337	1,970	8,114	4.8	878.8	878.8	879.5	0.7
AM	53,330	1,906	7,811	4.9	883.6	883.6	884.2	0.6

<sup>&</sup>lt;sup>1</sup>Feet above confluence with Gila River

1T	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA	
BLE	MARICOPA COUNTY, AZ		
6	AND INCORPORATED AREAS	CENTENNIAL WASH	

FLOODING	FLOODING SOURCE FLOOD			ODWAY BASE FLOOD WATER SURFACE ELEVA (FEET NAVD88)			TION	
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/ SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Centennial Wash (continued)								
AN	55,403	1,917	9,000	4.3	887.8	887.8	888.4	0.6
AO	57,225	1,929	8,403	4.6	891.9	891.9	892.6	0.7
AP	59,228	2,479	10,037	3.8	896.8	896.8	897.3	0.5
AQ	61,318	1,985	8,928	4.3	901.4	901.4	902.2	0.8
AR	63,224	1,836	9,344	4.1	906.7	906.7	907.7	1.0
AS	65,260	1,373	8,269	4.7	911.8	911.8	912.4	0.6
AT	67,446	1,859	9,968	3.9	916.6	916.6	917.2	0.6
AU	69,450	2,672	13,253	2.9	921.8	921.8	922.7	0.9
AV	71,448	2,319	12,746	3.0	925.7	925.7	926.5	0.8
AW	73,563	1,821	7,735	5.0	930.0	930.0	930.3	0.3
AX	75,200	1,047	7,049	5.5	935.3	935.3	935.8	0.5
AY	77,205	1,680	12,243	3.2	941.5	941.5	942.3	0.8
AZ	79,300	1,531	7,574	5.1	945.1	945.1	945.4	0.3
BA	81,380	1,385	7,269	5.3	949.8	949.8	950.4	0.6
BB	83,383	1,760	8,854	4.4	956.7	956.7	957.5	0.8
BC	85,401	1,128	5,048	7.6	961.7	961.7	962.0	0.3
BD	87,413	958	5,809	6.6	969.8	969.8	970.5	0.7
BE	89,463	1,144	4,986	7.7	975.2	975.2	975.6	0.4
BF	91,487	809	5,100	7.6	983.3	983.3	983.9	0.6
BG	93,461	767	5,022	7.7	990.6	990.6	990.7	0.1

<sup>&</sup>lt;sup>1</sup>Feet above confluence with Gila River

TA	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
BLE	MARICOPA COUNTY, AZ	
6	AND INCORPORATED AREAS	CENTENNIAL WASH

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Centennial Wash (continued)								
ВН	95,427	724	4,990	7.7	996.6	996.6	997.3	0.7
BI	97,453	731	4,823	8.0	1,003.7	1,003.7	1,004.4	0.7
BJ	99,473	783	5,794	6.7	1,010.6	1,010.6	1,011.0	0.4
BK	101,475	792	6,072	6.4	1,015.8	1,015.8	1,016.5	0.7
BL	103,489	933	6,654	5.8	1,021.7	1,021.7	1,022.1	0.4
BM	105,420	1,291	8,222	4.7	1,027.6	1,027.6	1,028.6	1.0
BN	107,498	1,244	8,582	4.5	1,034.9	1,034.9	1,035.7	0.8
ВО	109,492	918	7,314	6.0	1,042.5	1,042.5	1,042.8	0.3
BP	111,114	1,299	9,410	4.1	1,045.9	1,045.9	1,046.4	0.4
BQ	113,582	1,442	11,942	3.2	1,051.2	1,051.2	1,051.9	0.7
BR	115,204	1,407	11,069	3.7	1,053.5	1,053.5	1,054.2	0.7
BS	117,211	2,136	18,524	2.3	1,058.8	1,058.8	1,059.6	0.8
BT	119,208	2,749	20,557	1.9	1,061.4	1,061.4	1,062.3	0.9
BU	121,360	3,097	16,476	2.3	1,063.8	1,063.8	1,064.6	0.8
BV	123,358	2,858	9,649	2.3	1,066.0	1,066.0	1,066.6	0.6
BW	125,306	2,544	7,719	2.8	1,069.1	1,069.1	1,069.2	0.1
BX	127,394	2,265	8,697	2.5	1,076.0	1,076.0	1,076.6	0.6
BY	129,349	2,344	9,261	2.3	1,079.8	1,079.8	1,080.4	0.6
BZ	131,256	2,979	12,009	1.8	1,083.3	1,083.3	1,084.0	0.7
CA	133,382	2,226	11,397	1.9	1,088.0	1,088.0	1,088.9	0.9

<sup>1</sup>Feet above confluence with Gila River

Ţ.	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA	
ſ	MARICOPA COUNTY, AZ	123354741 574174	
	AND INCORPORATED AREAS	CENTENNIAL WASH	

FLOODING	FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Centennial Wash (continued)									
CB CC CD CE CF CG CH CI CJ CK CL CM	135,474 137,122 139,543 141,116 143,353 145,212 147,005 149,541 151,228 153,289 155,348 157,332 159,321	1,386 1,693 1,920 1,942 1,852 1,920 2,344 1,811 1,626 1,541 1,488 1,304 1,980	5,831 9,114 9,101 9,686 7,095 8,143 8,310 8,443 6,575 6,700 5,250 4,715 5,725	3.7 2.4 2.4 2.3 3.1 2.7 2.7 1.9 2.4 2.4 3.3 3.6 3.0	1,092.0 1,096.5 1,101.2 1,105.0 1,109.5 1,116.5 1,121.9 1,129.3 1,133.5 1,139.2 1,145.1 1,151.2	1,092.0 1,096.5 1,101.2 1,105.0 1,109.5 1,116.5 1,121.9 1,129.3 1,133.5 1,139.2 1,145.1 1,151.2	1,092.3 1,097.3 1,101.7 1,105.9 1,109.7 1,116.7 1,122.5 1,130.0 1,133.8 1,139.8 1,145.5 1,152.0 1,158.5	0.3 0.8 0.5 0.9 0.2 0.3 0.6 0.7 0.3 0.6 0.4 0.8 0.7	
CO CP CQ CR CS CT	161,362 163,216 165,332 167,445 169,214 171,584 173,276	3,161 3,125 3,013 2,512 2,441 2,820 2,934	7,900 8,676 7,031 4,902 7,020 9,290 9,803	2.6 2.3 2.9 4.1 2.9 2.5 2.4	1,164.9 1,170.7 1,176.7 1,182.1 1,188.8 1,196.0 1,201.6	1,164.9 1,170.7 1,176.7 1,182.1 1,188.8 1,196.0 1,201.6	1,165.4 1,171.6 1,177.5 1,182.4 1,189.7 1,196.8 1,202.5	0.5 0.9 0.8 0.3 0.9 0.8 0.9	

<sup>&</sup>lt;sup>1</sup>Feet above confluence with Gila River

1/	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA		
BLE	MARICOPA COUNTY,AZ			
6	AND INCORPORATED AREAS	CENTENNIAL WASH		

FLOODING SOURCE				BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)				
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Centennial Wash (continued)								
CV	175,210	2,429	5,337	4.4	1,207.2	1,207.2	1,207.7	0.5
CW	177,311	2,553	5,799	4.1	1,214.0	1,214.0	1,214.7	0.7
CX	179,183	3,045	7,823	4.4	1,220.6	1,220.6	1,221.4	0.8
CY	181,416	4,551	9,936	3.5	1,226.7	1,226.7	1,227.7	1.0
CZ	183,405	3,836	9,233	3.7	1,233.0	1,233.0	1,233.8	0.8
DA	185,355	3,419	10,735	3.2	1,239.2	1,239.2	1,240.0	0.8
DB	187,291	2,914	11,472	3.0	1,245.0	1,245.0	1,245.7	0.7
DC	189,502	2,085	9,825	3.5	1,251.7	1,251.7	1,252.4	0.7
DD	191,400	1,875	9,098	3.8	1,256.7	1,256.7	1,257.4	0.7
DE	193,397	1,730	9,436	3.6	1,262.0	1,262.0	1,262.8	0.8
DF	195,412	1,727	8,673	4.0	1,266.7	1,266.7	1,267.3	0.6
DG	197,228	1,519	8,550	4.0	1,271.3	1,271.3	1,272.0	0.7
DH	199,264	1,432	8,549	4.0	1,276.5	1,276.5	1,277.4	0.9
DI	201,408	1,832	7,827	4.4	1,282.4	1,282.4	1,282.9	0.5
DJ	203,541	2,154	8,318	4.1	1,289.3	1,289.3	1,289.7	0.4
DK	205,463	2,153	10,672	3.2	1,295.5	1,295.5	1,296.4	0.9
DL	207,227	1,826	9,641	3.6	1,300.7	1,300.7	1,301.5	0.8
DM	209,283	1,680	9,178	3.7	1,307.3	1,307.3	1,308.2	0.9
DN	211,329	2,061	9,381	3.7	1,313.5	1,313.5	1,314.2	0.7
DO	213,828	2,017	8,299	4.1	1,322.7	1,322.7	1,323.0	0.3

<sup>&</sup>lt;sup>1</sup>Feet above confluence with Gila River

1,	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA	
BLE	MARICOPA COUNTY, AZ		
6	AND INCORPORATED AREAS	CENTENNIAL WASH	

FLOODING	FLOODING SOURCE					BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Centennial Wash Field Overflow									
A B C D	1,170 1,993 3,181 4,324	5,232 5,304 4,749 707	17,180 16,398 14,049 19,347	2.8 2.3 2.4 0.6	843.7 845.4 847.9 851.8	843.7 845.4 847.9 851.8	843.8 845.4 847.9 851.8	0.1 0.0 0.0 0.0	

<sup>&</sup>lt;sup>1</sup>Feet above confluence with Centennial Wash

Τ́	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA		
BLE	MARICOPA COUNTY, AZ	12000		
ი ი	AND INCORPORATED AREAS	CENTENNIAL WASH FIELD OVERFLOW		

FLOODING SOURCE		FLOODWAY		BASE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)				
CROSS SECTION	DISTANCE <sup>1</sup>	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Centennial Wash West Railroad Overflow								
А	3,165	599	2,004	2.5	851.8	849.5 <sup>2</sup>	849.5 <sup>2</sup>	0.0
В	4,039	408	1,591	2.4	852.3	852.3	853.0	0.7
С	5,101	540	2,940	3.4	853.3	853.3	854.2	0.9
D	6,039	691	3,342	3.0	855.0	855.0	855.6	0.6
Е	7,044	833	4,433	2.3	856.0	856.0	856.8	0.8
F	8,040	1,117	6,850	3.3	857.1	857.1	857.9	0.8
G	9,241	2,146	12,265	1.8	857.6	857.6	858.5	0.9

<sup>&</sup>lt;sup>1</sup>Feet above confluence with Centennial Wash

[ -	FEDERAL EM	ERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
ΙĒ	≝ MAR	MARICOPA COUNTY, AZ	1200DWAT DATA
	0	INCORPORATED AREAS	CENTENNIAL WASH WEST RAILROAD OVERFLOW
	AND	INCOM ONATED ANEAS	

<sup>&</sup>lt;sup>2</sup>Elevation computed without consideration of backwater effects from Centennial Wash Field Overflow